

# Stress

Traction  $\vec{T}$   
may depend on

Body  $f$   
type  
material

or  
 $\frac{d}{dt}$   
time  $t$

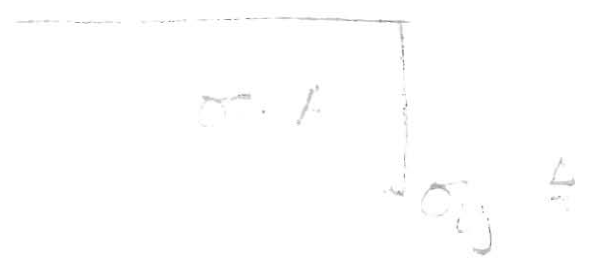
- Forces inside body reaction to traction are a
- Homogeneous of traction at

Then traction on each face of

Force to be static  
(+2 face) = (-2 face) and +2 face

traction  $\sigma / a$

$\Rightarrow \sigma \cdot A$   
stress / area normal  $\Rightarrow +$

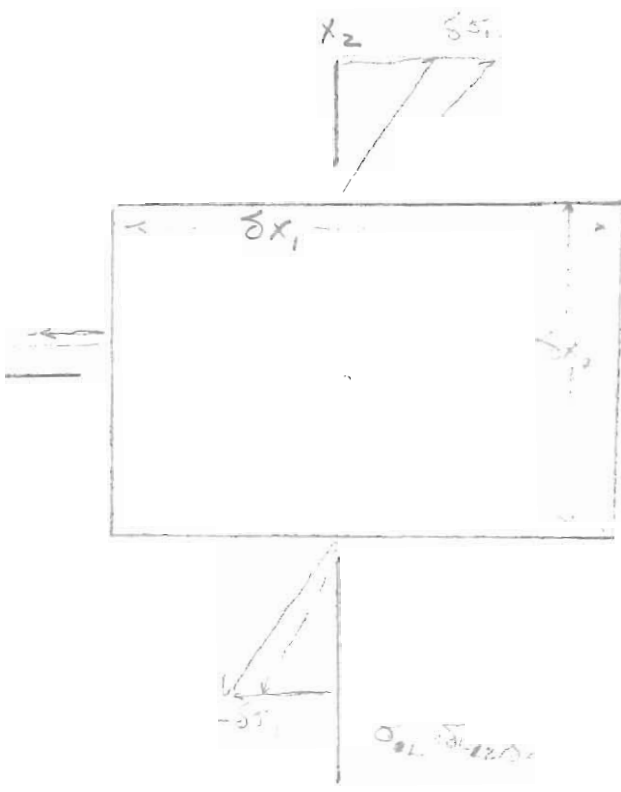


$\sigma$  + if to

$\sigma_{11} = \sigma_{22}$

| if

Eqn. Equilibrium



$$+ \sigma_{11} \delta x_1 \delta x_2 + \frac{\partial \sigma_{11}}{\partial x_1} \delta x_1 \delta x_2 \delta x_2$$

$$- \left[ \sigma_{11} \delta x_2 \delta x_2 - \frac{\partial \sigma_{11}}{\partial x_1} \delta x_1 \delta x_2 \delta x_2 \right]$$

result  $\frac{\partial \sigma_{11}}{\partial x_1} \delta x_1 \delta x_2$

on  $x_2$  faces

$$-\sigma_{11}$$

$$\frac{\partial \sigma_{12}}{\partial x_2} \delta x_1 \delta x_2$$

B. 1

sum of forces

$$\frac{\partial \sigma_{11}}{\partial x_1} \delta x_1 \delta x_2 + \frac{\partial \sigma_{12}}{\partial x_2} \delta x_1 \delta x_2 + \frac{\partial \sigma_{13}}{\partial x_3} \delta x_1 \delta x_2 \delta x_3 = \rho \delta V$$

in general,

$$\frac{\partial \sigma_{ij}}{\partial x_j} + \rho g_i = \rho x_i$$

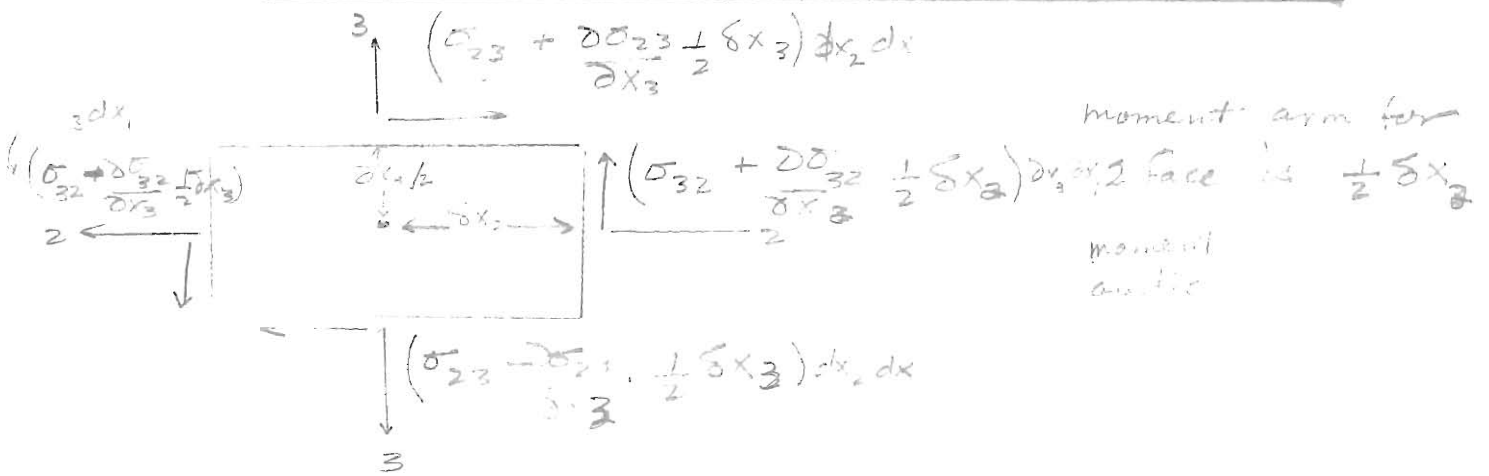
If

$$\frac{\partial \sigma_{ij}}{\partial x_j} + \rho g_i = 0$$

static eq

# 3 of 6

## Shear moments and symmetry



moment arm for 3 face is  $\frac{1}{2} dx_3$   
 moments are close

$$2\sigma_{32} \frac{dx_3}{2} (dx_2 dx_1) - 2\sigma_{23} \frac{dx_2}{2} dx_3 dx_1 + G_1 dx_1 dx_2 dx_3 = I_1 \Theta_1$$

but assume no torsion  $G_1 = 0$

and note  $I_1$  ord of mag  $\propto dx^5 \rightarrow 0$  as  $dx \rightarrow 0$

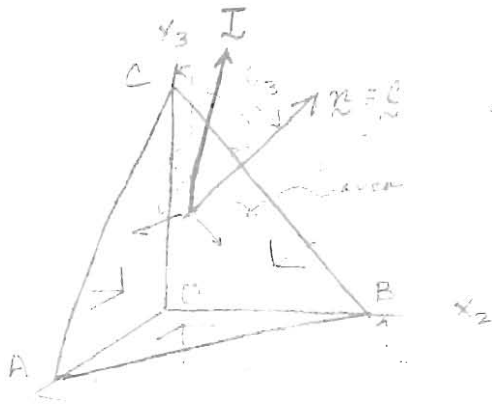
then  $(\sigma_{32} - \sigma_{23}) dx_1 dx_2 dx_3 = 0$

$$\Rightarrow \sigma_{32} = \sigma_{23}$$

$$\text{or } \sigma_{ij} = \sigma_{ji}$$

$\Rightarrow$  Stress tensor symmetric

a tensor



Proof that stress

Cauchy tet. is

Total force on face ABC

$$\underline{T} = \underline{P} \cdot (\underline{ABC})$$

area ABC

$$P_1(\underline{ABC}) = \sigma_{11}(BDC) + \tau_{12}(\underline{A})$$

$$P_1 = \sigma_{11} l_1 + \tau_{12} l_2 + \tau_{13} l_3$$

where  $l =$

$$l = \begin{bmatrix} l_1 \\ l_2 \\ l_3 \end{bmatrix}$$

$$\text{Sim. } P_2 = \tau_{21} l_1 + \sigma_{22} l_2 + \tau_{23} l_3$$

$$P_3 = \tau_{31} l_1 + \tau_{32} l_2 + \sigma_{33} l_3$$

$$\text{or } P_i = \sigma_{ij} l_j$$

i.e. tensor transformation rule

Stress is a 2nd rank symmetric tensor

⇒ Principal stresses

Quadratic



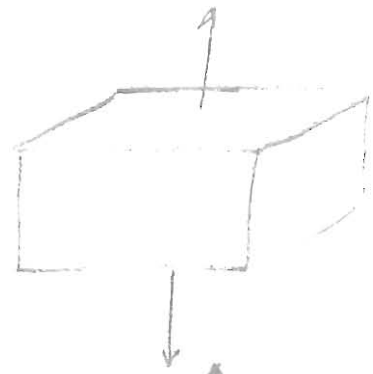
$$\begin{bmatrix} \sigma_{11} & \sigma_{12} & \sigma_{13} \\ \sigma_{21} & \sigma_{22} & \sigma_{23} \\ \sigma_{31} & \sigma_{32} & \sigma_{33} \end{bmatrix}$$

$$\begin{bmatrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_2 & 0 \\ 0 & 0 & \sigma_3 \end{bmatrix}$$

# Special Forms of Stress 5 of 6

i.) uniaxial stress

$$\begin{bmatrix} \sigma & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$



Ref: Ch 1

ii.) biaxial stress

$$\begin{bmatrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_2 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

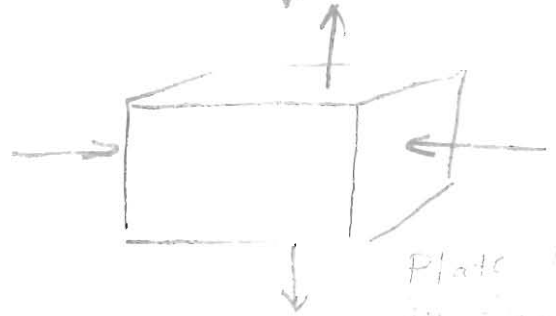


Plate under  
eq. forces & out  
at end  
 $\sigma_1, \sigma_2 \ll \sigma_3$

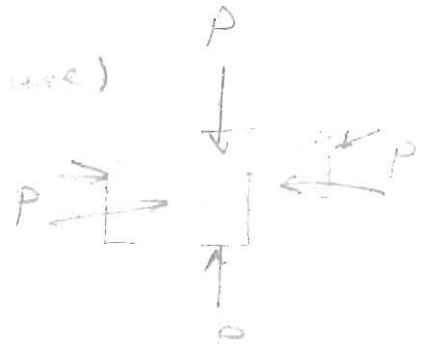
iii.) triaxial stress

$$\begin{bmatrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_2 & 0 \\ 0 & 0 & \sigma_3 \end{bmatrix}$$



iv.) hydrostatic stress (pressure)

$$\begin{bmatrix} -p & 0 & 0 \\ 0 & -p & 0 \\ 0 & 0 & -p \end{bmatrix} = -p \delta_{ij}$$



v.) Pure shear

$$\begin{bmatrix} -\sigma & 0 & 0 \\ 0 & \sigma & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

vi.) Simple shear

$$\begin{bmatrix} 0 & \sigma & 0 \\ \sigma & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$



## Summary Stress Tensor

1.) Traction on a plane with direction cosine  $\underline{\underline{l}}$

$$\underline{\underline{T}} = \underline{\underline{\sigma}} \underline{\underline{l}} \quad T = \sigma_{ij} l_j$$

2.) Equilibrium

$$\frac{\partial \sigma_{ij}}{\partial x_i} + \rho g_j = m \ddot{x}_j$$

3.) Stress is symmetric

$$\sigma_{ij} = \sigma_{ji}$$

4.) Principal stress directions  
values

$$\begin{matrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_{II} & 0 \\ 0 & 0 & \sigma_{III} \end{matrix}$$

5.) Stress quadratic construction

Stress, Strain

Elasticity:

1 of 8

Nye Chap.

§ 6, pp 82-105.

### 1. One dimensional stress

Relative displacement increment



$\Delta u$

$b$

$x$

$P$  is

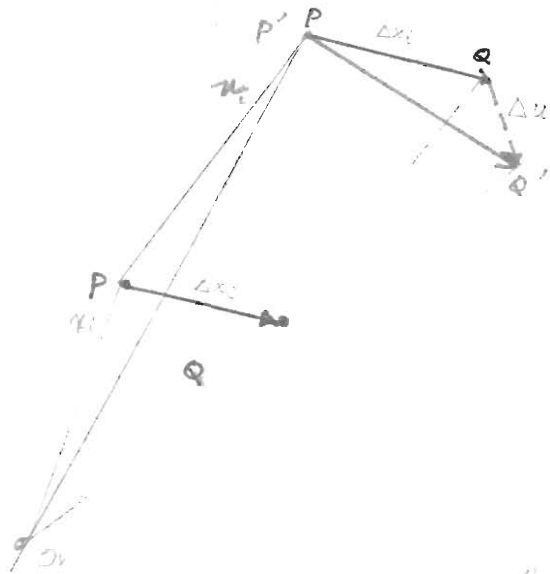
$$= P' \frac{\Delta u}{\Delta x}$$

where,

Goal: For a given deformation of body, define a tensor that describes the change of direction and length of any vector in body.

Want to map vector in body (undef.) into vector in body (def.)

# Infinitesimal Strain - (2 of 3)



Point P can be written as  $P'$

$$[OP] = x_i$$

$$[OP' - P] = u_i \quad (\text{translation})$$

Consider a line PQ

$$[PQ] = L x_i$$

Point Q is written as Q

are not zero.

$$\Delta PQ = \overline{QQ'} = P'Q' - PQ = \Delta u$$

but for any continuous diff. variable.

$$\Delta u = \frac{\partial u_i}{\partial x_j} L x_j$$

$$\Delta u = e_{ij} \Delta x_j$$

Define  $e_{ij} = \frac{\partial u_i}{\partial x_j}$

Displacement  
Gradient Tensor



171 2

Statements

1.)  $e_{ij}$  is a second rank tensor

$$u_i = a_{ij} u'_j$$

old

$$\Rightarrow \frac{\partial}{\partial x'_i} = \frac{\partial}{\partial x_k} \frac{\partial x_k}{\partial x'_i} = a_{jk} \frac{\partial}{\partial x_k}$$

then

$$\frac{\partial u'_i}{\partial x'_j} = a_{ik} a_{jl} \frac{\partial u_k}{\partial x_l}$$

$$\Rightarrow e'_{ij} = a_{ik} a_{jl} e_{kl}$$

2.)

Define tensor  $\delta_{ij}$  as

Strain tensor

not  
can

1

0

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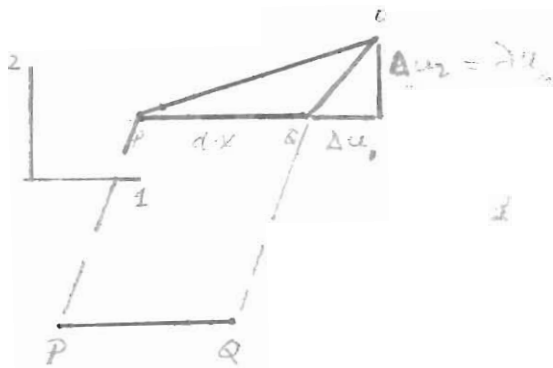
1

# Specific Examples of strain

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Now consider a special case

$$PQ = \hat{e}_1 dx \quad \text{where } \hat{e}_1 \text{ unit vector along } x_1 \text{ axis}$$



$$\Delta u_i = \frac{\partial u_i}{\partial x_j} \Delta x_j$$

$$\Delta u_1 = \frac{\partial u_1}{\partial x_1} \Delta x_1 + \frac{\partial u_1}{\partial x_2} \Delta x_2 + \frac{\partial u_1}{\partial x_3} \Delta x_3$$

by definition  $\Delta u_i = e_{i1} \Delta x_1 + e_{i2} \Delta x_2 + e_{i3} \Delta x_3$

$e_{11}$  is change of length of  $dx_1 \vec{e}_1$  in 1 direction  
normal strain note:  $\frac{L + \Delta L}{L} = \text{stretch} = 1 + e_{11}$

$e_{12}$  is change of length of  $dx_1 \vec{e}_1$  in 2 direction  
 shear strain

note  $\frac{\partial u_2}{\partial x_1} dx_1 / dx_1 + \frac{\partial u_1}{\partial x_2} dx_2 = \tan \theta$

but  $\theta \ll 1$  so  $\tan \theta \approx \theta$

and  $\frac{\partial u_2}{\partial x_1} \ll 1$  so  $dx_2 \frac{\partial u_1}{\partial x_2} dx_2 \approx dx_1$

$$\Rightarrow \frac{\partial u_2}{\partial x_1} = e_{21} \approx \theta$$

SO  $e_{11}$  measure change of length in 1 direction of a line in 1 direction

$e_{12}$  measures change of length in 1 dir of a line in 2 direction originally

Infinitesimal Strain

## Example 2

Suppose rigid body rotation:  
 consider rotation of vectors  $\hat{e}_1$  and  $\hat{e}_2$   
 for  $\hat{e}_2$   $\frac{\partial u_1}{\partial x_2} = -\theta$  because  $\partial u_1 < 0$

from definition

$$\omega_{12} = \frac{1}{2} \left( \frac{\partial u_1}{\partial x_2} - \frac{\partial u_2}{\partial x_1} \right)$$

$$= \frac{1}{2} (-\theta - +\theta) = -\theta$$

$$\text{but } \epsilon_{12} = \frac{1}{2} (\epsilon_{12} + \epsilon_{21}) = 0$$

Particular Special Types of StrainHomogenous strain:

All elements in body strained same

$$\epsilon_{ij} \neq \epsilon_{ij}(x_1, x_2, x_3)$$

st lines  $\rightarrow$  st lines

p// line  $\rightarrow$  p// lines

all st. lines

ext. cont

by some ratio

an ellipse becomes a diff. ellipse

" circle

"

ellipse

Heterogenous Strain

$$\epsilon_{ij} = \epsilon_{ij}(x_k)$$

# Strain

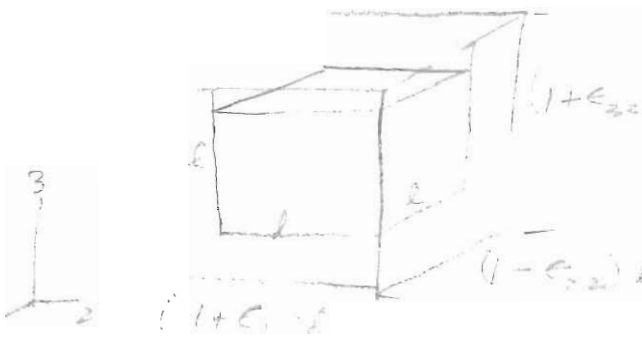
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## Plane Strain

All elements in a plane in original remain in a plane in deformed body (actually impossible) but nearly achieved if  $\epsilon_{33} \approx 0$  and  $\epsilon_{11}, \epsilon_{22}$  very large compared to  $\epsilon_{33}$  dimensions



## Volumetric Strain



$$l^3 (1 + \epsilon_{11})(1 + \epsilon_{22})(1 + \epsilon_{33}) \approx l^3 (1 + \epsilon_{11} + \epsilon_{22} + \epsilon_{33} + \dots)$$

fractional change in vol

$$\frac{\Delta V}{V} = \frac{l^3 (1 + \epsilon_{11} + \epsilon_{22} + \epsilon_{33}) - l^3}{l^3}$$

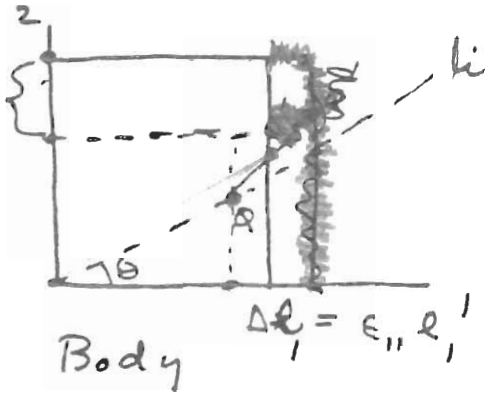
$$= \epsilon_{11} + \epsilon_{22} + \epsilon_{33}$$

(But remember trace of tensor is one of the

IB3C2

Example:

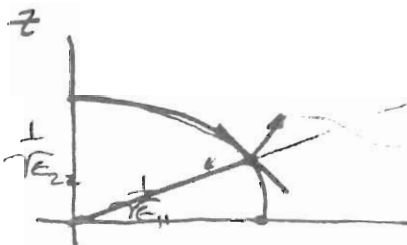
$$\Delta l_2 = \epsilon_{22} l_2$$



No shear along axes  
 $\Rightarrow$  these must be principal axes

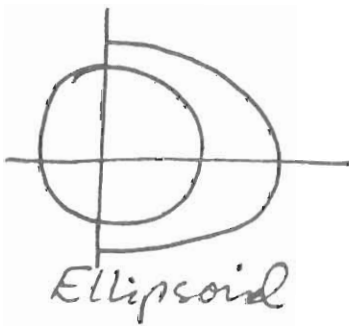
Assume  $\epsilon_{11} < \epsilon_{22}$

$$\frac{1}{\gamma \epsilon_{11}} > \frac{1}{\gamma \epsilon_{22}}$$



$$\Delta l_i = \epsilon_{ij} l_j$$

Quadratic



Ellipsoid

Equation of circle =  $x^2 + y^2 = 1$

Equation of deformed ellipsoid

$$x \rightarrow (1 + \epsilon_1) x = x'$$

$$\frac{x'}{1 + \epsilon_1} = x$$

$$\Rightarrow \frac{x'^2}{(1 + \epsilon_1)^2} + \frac{y'^2}{(1 + \epsilon_2)^2} + \frac{z'^2}{(1 + \epsilon_3)^2} = 1$$

Homework:

Exercise 6.1 in Nye: A small def. is defined by the tensor

$$\epsilon_{ij} = \begin{bmatrix} 8 & -1 & 1 \\ 1 & 6 & 0 \\ -5 & 0 & 2 \end{bmatrix} \times 10^{-6}$$

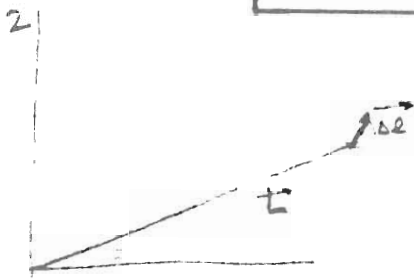
Find magnitudes of principal

## Summary: Strain

Strain is the symmetric part of the deformation gradient tensor

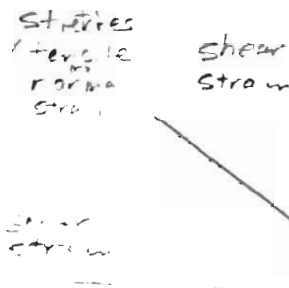
$$\epsilon_{ij} \triangleq \frac{1}{2} [e_{ij} + e_{ji}]$$

$$\equiv \frac{1}{2} \left[ \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right]$$



$$\Delta L \approx \epsilon L$$

$$\Delta L_i = \epsilon_{ij} L_j = \left( \frac{1}{2} \left( \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right) \right) L_j$$



Mohr's circle constructor applies for 2d principal strain

Also because symmetric 2nd rank tensor

⇒ 3 principal directions

3 principal values

stretch in an arbitrary direction

$$\Delta L_i = \epsilon_{ij} (L \cdot \vec{e}_j) = \epsilon_{ij} a_j |L|$$

$$\lambda = \frac{\Delta L_i}{|L|} = \epsilon_{ij} a_j \text{ where } a_j \text{ is } \cos(\angle \text{between } \vec{e}_j \text{ and } L)$$

Reading : Creep of crystals, Poirier, Chap 1.

Work :

$$\underbrace{\sigma_{ij} d\epsilon_{ij}}_{\text{full stress tensor}} \quad \text{or} \quad \underbrace{PdV}_{\substack{\text{"isotropic"} \\ \text{hydrostatic} \\ \text{lithostatic}}} \quad \underbrace{\sigma_{ij} - P\delta_{ij}}_{\substack{\text{non-hydrostatic} \\ \downarrow \text{deviatoric stress}}} \\ \text{differential stress} \\ \sigma_I - \sigma_{III}$$

② In general, deformation occurring by crystal plastic mechanisms responds to differential stress or non-hydrostatic stress.

① When material is not porous, then  $\Delta V_{\text{permanent}} = 0$  when crystal plasticity (i.e. vacancies, dislocations, twinning, gb. sliding)  
[Exception is phase change]

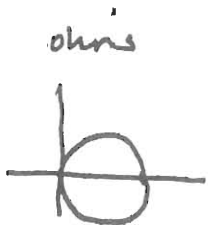
⑥ For elastic processes, all work converted potential energy of separation of bonds. Recoverable (Egn. of State)

③ Effect of Intermediate Stress:

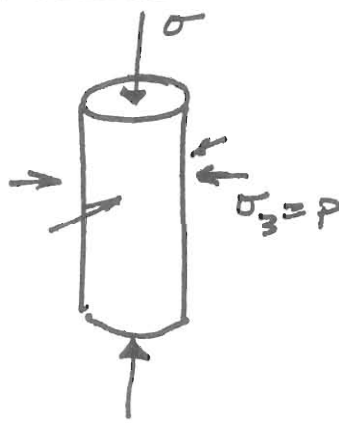
$$\sigma_{\text{OCTAHEDRAL}} = \sqrt{(\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2}$$

$$\sigma_{\text{DIFFERENTIAL}} = \sigma_I - \sigma_{III}$$

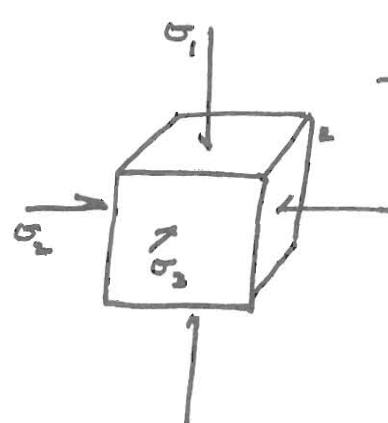
# Mechanical Tests:



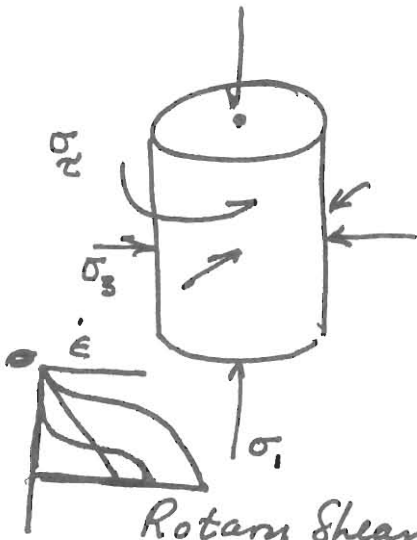
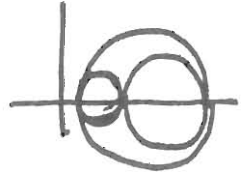
uniaxial



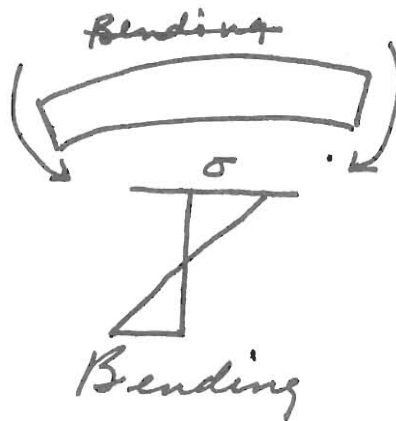
"conventional triaxial"



true triaxial



Rotary Shear



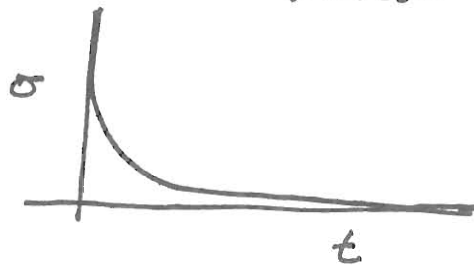
Bending

## Uniaxial $\dot{\epsilon}$ Conv. triaxial (Boundary Conditions)

Creep tests : constant load (or const.  $\sigma$ )

Constant  $\dot{\epsilon}$  : constant displ. rate (or const.  $\dot{\epsilon}$ )

Relaxation : Load to yield; stop crosshead wait for relaxation





## Observed Variables:

$$T, P, l_s(t), (d_s(t)), \text{Load}(t)$$

T → thermocouple

P → pressure transducers

l → displacement transducers DCDT or VDT

L → load cell (displacement in elastic element)

- convert displacement to load using Hooke's law  
convert load to stress using calculated area  
or measured area

- Calculate stress

$$\text{Load} / \text{Area}(t)$$

Calculate area (1. measurement

2. assume no volume change

3. interpolate from initial to

$$\text{typical } \frac{T}{0 - 1600 \text{ K}}, \frac{P}{(0 - 1000 \text{ mpa})}, \frac{\dot{\epsilon}}{10^{-6} - 10^{-8}} \Rightarrow 10^6 \text{ secs}$$

$$(0 - 50 \text{ GPa})$$

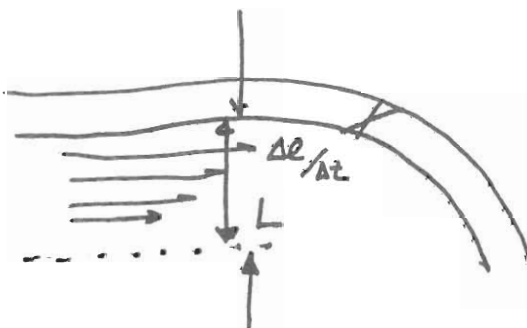
## Natural Boundary Conditions

constant displacement rate - descending plate

constant load

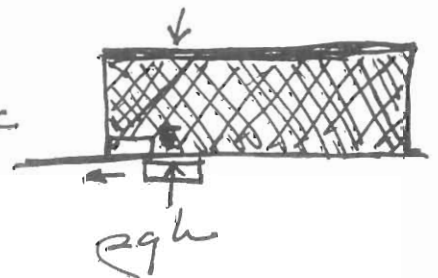
- mountain

Olympus Mons



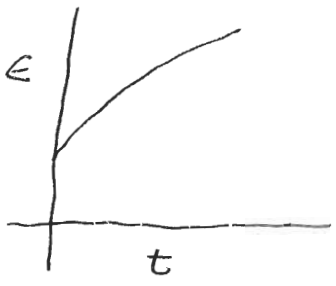
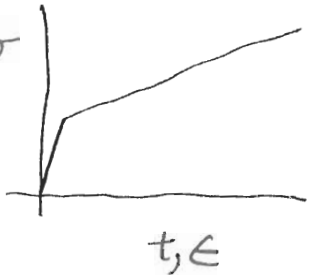
$$\frac{10^{-2} \text{ (1 cm)}}{3 \cdot 10^{27} \text{ (1 yr)}} = 2 \cdot 10^{-16} / \text{sec}$$

$$\frac{100 \cdot 100 \cdot 10^3}{1.5 \cdot 10^3 \cdot 10^3} \text{ (whole mantle)}$$



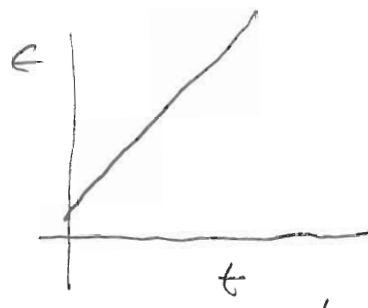
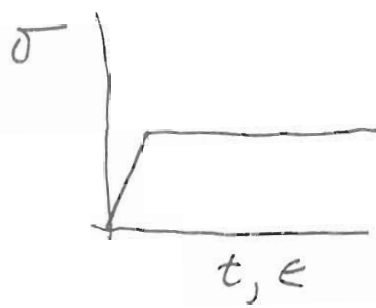
# Macroscopic Mech. Behavior

work  
hardening



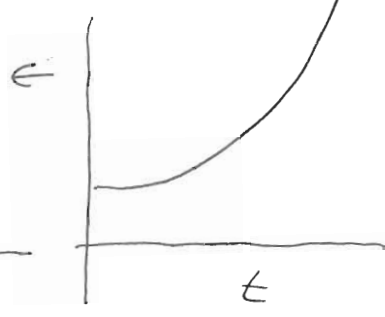
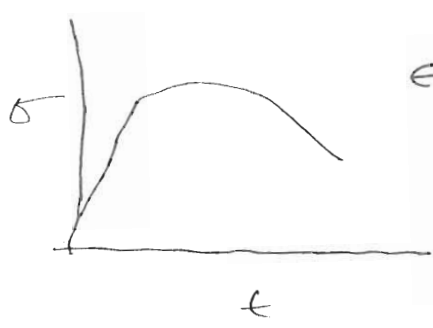
decelerating  
creep rate

steady  
state



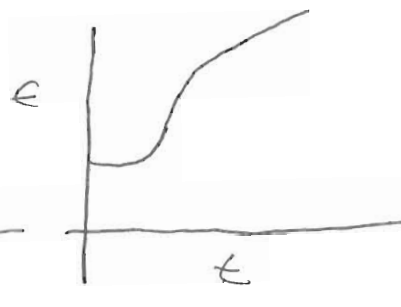
st. st

work  
softening



accel.  
creep

yield  
pt



Sigmoidal  
creep  
curve

## Stability of deformation :

instability - material props  
loading conditions  
geometric

$$\Delta E \Rightarrow \Delta T_{\text{bud}} < 0$$

## Constitutive Law:

- Predict strength or deformation rate using observable quantities at a given instant in time. (i.e. thermodynamic state variables that do not depend on path)  
⇒ strain & time specifically excluded.

$$\dot{\epsilon} = f(\sigma, T, P, f_{ci}, \gamma(\epsilon, t), \dots)$$

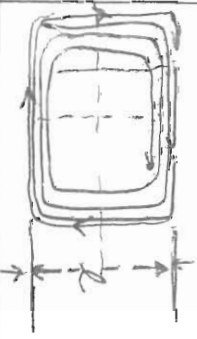
$\gamma(\epsilon, t)$  - e.g. defect # (extrinsic chemistry)  
geometry

$\gamma(\epsilon, t)$  may not be a function of  $\epsilon, t$   
should be determined by separate observation.

$\gamma(\epsilon, t)$  internal state variable

- For fixed "external" state variables  
internal variable might change  
instability, weakening, or hardening

# Lab of Natural Loading Conditions

$\sigma$	P	T	$\dot{\epsilon}$
<p><math>0 - 100 \text{ MPa}</math></p> <p><math>0 - 1000 \text{ MPa}</math></p>	<p><math>0 - 600 \text{ MPa}</math></p>	<p><math>0 - 1600 \text{ K}</math></p>	<p>101 in 1 day</p> <p>1 week</p> <p>1 month</p> <p>1 yr</p> <p><math>10^{-7}/s</math></p> <p><math>10^{-8}/s</math></p> <p><math>5 \cdot 10^{-10}/s</math></p> <p><math>5 \cdot 10^{-11}/s</math></p>
<p><math>0 - 500 \text{ MPa}</math></p> <p><math>(3)</math></p>	<p>50km ~ 175MPa</p> <p>100km ~ 320MPa</p> <p>IC/OC <math>0.3 \cdot 10^{11} \text{ Pa}</math></p> <p><math>0.3 \cdot 10^{12} \text{ Pa}</math></p>	<p><math>0 - 200^\circ \text{ / km}</math></p> <p>@ 95km 1400°C</p> <p>1600K @ 95km</p>	<p><math>\frac{1 \text{ cm}}{\text{yr}} \left( \frac{1}{300 \text{ km}} \right) \sim 10^{-15} / s</math></p> 

Lab

next

$R_c = 3473 \text{ km}$

$R_m = 6338$

$\Delta R_c = 0 - 50 \text{ km}$

1 Mb =  $10^{11} \text{ Pa}$

1 kb =  $10^8 \text{ Pa}$

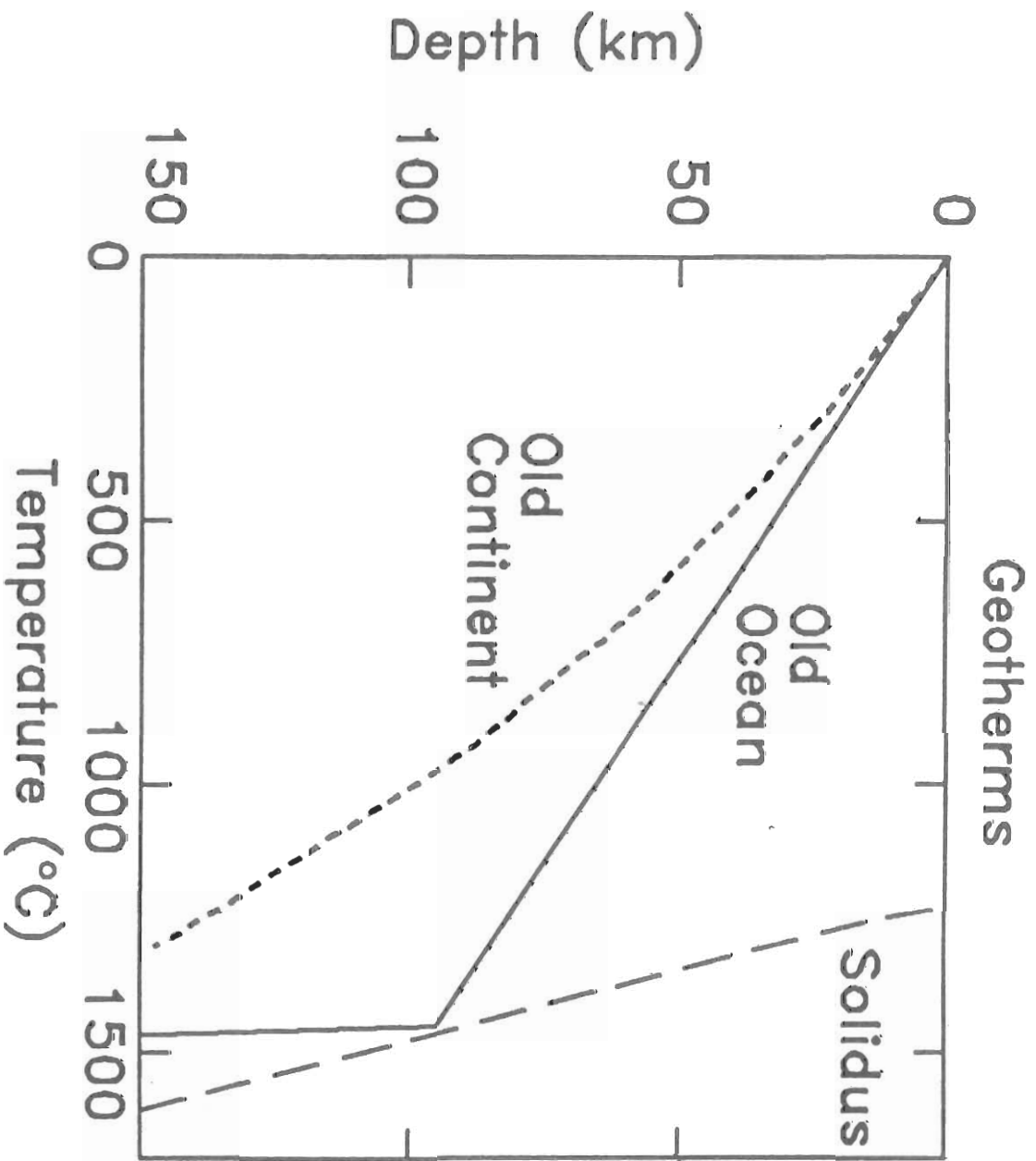


Fig. 5: Geotherms for old oceanic [105] and old con-