13.122 Lecture 1
Primary Load: Bending Moment and Shear Force

Introduction to course:
Design process
Structural design process

General course content:

13.122 Ship Structural Design

A. Loads on ship/offshore platforms
   Calculation of loads
   buoyancy, shear, bending moment
   "hand" using excel

B. Review of bending, shear and torsion - open sections

C. Modeling a structure
   Maestro
   checking loads and moments

D. Development of limit states and failure modes
   stress analysis of ship/ocean system structure

E. Design of section for bending
   project

F. Matrix analysis (Grillage), FEM Introduction

Expected outcome: an ability to effectively use structural design tools with an understanding of the underlying analysis.

Changes from previous years:
reduced 13.10 review (2002)
calculation of loads (bending moment and shear force) (2001)
introduce Maestro earlier for modeling and load analysis (2001)
introduce open section torsion
revert to earlier problem sets for mathcad limit state analysis (2001)
Sign Convention

In general, we will use "structural" sign convention described in Shames: 10.2 page 286

An axial force or bending moment acting on a beam cross-section is positive if it acts on a positive face and is directed in a positive coordinate direction. The shear force is positive if it acts in the negative direction on a positive face.

(positive face defined by outward normal in positive coordinate direction)

Moment of inertia is defined relative to the axis for measuring distance: \( I_z = \int y^2 \, dA \); \( I_y = \int z^2 \, dA \)

\[
\sigma_x = \tau_{xx} \\
\tau_{xy} = \tau_{yx} \\
\sigma_x = -\frac{M_z y}{I_z}
\]

define \( f \) = load, positive in +y direction,

\[ dQ = f \, dx \]

\[ Q = \int_0^x f(x) \, dx + Q(x = 0) = \int_0^x f(x) \, dx \]

moments around right face \( \Rightarrow \)

\[ M + Q \, dx + f(x) \, dx \frac{dx}{2} - M + dM = 0 \quad \Rightarrow \quad Q = \frac{d}{dx} M \]

\[ M(x) = \int_0^x Q(x) \, dx + M(x = 0) = \int_0^x Q(x) \, dx \]
Part 1: Calculation of loads
buoyancy, shear, bending moment

a) shear and bending moment from distributed force per length
b) shear and bending moment from point force
c) algorithms for calculation

a) shear and bending moment from distributed force per length

suppose we have a distribution of weight per foot \( w \), constant over a distance \( \xi_0 \Rightarrow \xi_1 \)

mathcad has an expression that handles the < and > relationships:
\[ a > b = 1 \text{ if true, 0 if false} \]

\[ a := 1 \quad b := 2 \quad a > b = 0 \quad b > a = 1 \]

for example: for \( i = 3 \) and \( x := 0, 0.1 \ldots 6 \)

\[ w_{\text{plot}}(x) := w_i \cdot (\xi_i, 0 < x \leq \xi_i, 1) \]

by superposition: for \( i := 0 \ldots 4 \)

\[ w_{\text{plot}}(x) := \sum_{i = 0}^{4} w_i \cdot (\xi_i, 0 < x \leq \xi_i, 1) \]
what is shear force from $w_i$?

\[
\text{shear}(x) := \int_0^x w_i(\xi) \, d\xi = \begin{cases} 
0 & \xi_i,0 > x \\
w_i(x - \xi_i,0) & \xi_i,0 < x \leq \xi_i,1 \\
w_i(\xi_i,1 - \xi_i,0) & x > \xi_i,1 
\end{cases}
\]

for example: $i := 2$

\[
\text{shear}(x) := w_i(x - \xi_i,0)(\xi_i,0 < x \leq \xi_i,1) + w_i(\xi_i,1 - \xi_i,0)(x > \xi_i,1)
\]

again by superposition: $i := 0..4$ using lower limit $= ll := 0$ and upper limit $= ul := 4$

\[
\text{shear}(x) := \sum_{i = ll}^{ul} \left[ w_i(x - \xi_i,0)(\xi_i,0 < x \leq \xi_i,1) + w_i(\xi_i,1 - \xi_i,0)(x > \xi_i,1) \right]
\]

for later comparison, let's rename this \( \text{shear}_1(x) := \text{shear}(x) \)

note that shear curve starts and ends at 0; why??

\[
\sum_i w_i(\xi_i,1 - \xi_i,0) = 0
\]
now what is bending moment from weight per foot as above?

\[ \text{bending}_\text{moment}(x) := \int_{0}^{x} \text{shear}(\xi) \, d\xi = \]

\[ \int_{0}^{x} \left[ w_i \left( x - \xi_i, 0 \right) \left( \xi_i, 0 < x \leq \xi_i, 1 \right) + w_i \left( \xi_i, 1 - \xi_i, 0 \right) \left( x > \xi_i, 1 \right) \right] \, d\xi \]

\[ = 0 \quad \text{when} \quad x < \xi_i, 0 \]

\[ = w_i \left[ \frac{x^2}{2} - \xi_i, 0 \cdot x \right] - \left\lbrack \frac{(\xi_i, 0)^2}{2} - (\xi_i, 0)^2 \right\rbrack \quad \text{when} \quad \xi_i, 0 < x < \xi_i, 1 \quad \text{or} \quad : = \frac{w_i}{2} \left( x - \xi_i, 0 \right)^2 \quad \text{after simplifying} \]

\[ = \left\lbrack \frac{w_i}{2} \left( x - \xi_i, 0 \right)^2 \right\rbrack \left( \xi_i, 0 < x \leq \xi_i, 1 \right) \quad \text{using the notation above.} \]

\[ = w_i \left[ \left( \xi_i, 1 - \xi_i, 0 \right) \left( x - \xi_i, 1 \right) + \frac{1}{2} \left( \xi_i, 1 - \xi_i, 0 \right) \left( \xi_i, 1 - \xi_i, 0 \right) \right] \left( x > \xi_i, 1 \right) \quad \text{when} \quad x > \xi_i, 1 \]

\[ \text{simplifying} \Rightarrow \quad \left\lbrack \frac{x^2}{2} - \xi_i, 0 \cdot x \right\rbrack - \left\lbrack \frac{(\xi_i, 0)^2}{2} - (\xi_i, 0)^2 \right\rbrack = \frac{x^2}{2} - \xi_i, 0 \cdot x + \frac{(\xi_i, 0)^2}{2} = \frac{1}{2} \left( x - \xi_i, 0 \right)^2 \]

therefore; bending moment \((x)\) = for lower limit: \( ll := 0 \) and upper limit = \( ul := 4 \)

\[ \text{bending}_\text{moment}(x) := \sum_{i=ll}^{ul} \left[ w_i \left[ \frac{1}{2} \left( x - \xi_i, 0 \right)^2 \right] \left( \xi_i, 0 < x \leq \xi_i, 1 \right) \ldots \right. \]

\[ \left. + w_i \left[ \left( \xi_i, 1 - \xi_i, 0 \right) \left( x - \xi_i, 1 \right) + \frac{1}{2} \left( \xi_i, 1 - \xi_i, 0 \right) \left( \xi_i, 1 - \xi_i, 0 \right) \right] \left( x > \xi_i, 1 \right) \right] \]

for later comparison we will save this as \( \text{bending}_\text{moment}_1 \,(x) := \text{bending}_\text{moment}(x) \)
b) shear and bending moment from point force

now consider if load is concentrated at a point between \(\xi_{i,0}\) and \(\xi_{i,1}\):

define \( f_i := w_i (\xi_{i,1} - \xi_{i,0}) \) when \( x = x_i \), i.e. and we will define \( x_i := \frac{1}{2} (\xi_{i,1} + \xi_{i,0}) \)

this is not necessary as \( x_i \) can be at any position (but usually between the end points).
In this case \( \text{shear}(x) := f_i \{ x \geq x_i \} \)

for example: \( i := 2 \) \( x_i = 3 \) \( \text{shear}(x) := f_i \{ x \geq x_i \} \)

as above total shear along beam with \( l_l := 0 \) and \( u_l := 4 \) \( \Rightarrow \) \( \text{shear}(x) := \sum_{i = l_l}^{u_l} f_i \{ x \geq x_i \} \)

let's compare this version with the earlier weight per foot \( \text{shear}_1(x) \)
now for bending moment: \( \text{bending}_\text{moment}(x) := \int_0^x \text{shear}(\xi) \, d\xi \) as above let’s look first at bending moment from each component: \( \text{shear}(x) := f_i(\{x \geq xx_i\}) \)

\[
\begin{align*}
\text{bending}_\text{moment}(x) &:= \int_0^x f_i(\{x \geq xx_i\}) \, d\xi \\
&= \text{bending}_\text{moment}(x) := f_i(\{x - xx_i\}) \{x \geq xx_i\}
\end{align*}
\]

plotting shear and bending moment for \( i := 0 \); \( \text{shear}(x) := f_i(\{x \geq xx_i\}) \) and \( \text{bending}_\text{moment}(x) := f_i(\{x - xx_i\}) \{x \geq xx_i\} \)

and as above total bending moment is the superposition of all components:

\[
\begin{align*}
\text{bending}_\text{moment}(x) &:= \sum_{i=\mathbb{I}}^{\mathbb{U}} f_i(\{x - xx_i\}) \{x \geq xx_i\}
\end{align*}
\]

again comparing with the weight per foot from above.
this doesn't look all that great, but we let a constant 3 span two intervals, what if we split it into two parts?

weight per foot between $\xi_{i,0} \Rightarrow \xi_{i,1}$

\[
\begin{align*}
\mathbf{w} &:= \begin{pmatrix} 1 \\ 2 \\ -3 \\ -3 \\ 2 \\ 1 \end{pmatrix} \\
\xi &:= \begin{pmatrix} 0 & 1 \\ 1 & 2 \\ 2 & 3 \\ 3 & 4 \\ 4 & 5 \\ 5 & 6 \end{pmatrix} \\
\mathbb{X}_i &:= \frac{\xi_{i,1} + \xi_{i,0}}{2} \quad \mathbb{X} = \begin{pmatrix} 0.5 \\ 1.5 \\ 2.5 \\ 3.5 \\ 4.5 \\ 5.5 \end{pmatrix}
\end{align*}
\]

bending_moment(x) := \sum_{i = \text{ll}}^{\text{ul}} f_i(x - \text{xx}_i)(x \geq \text{xx}_i) \\
f_i := w_i(x_i, 1 - x_{i,0})

notes_10_shear_bending.mcd
c) algorithms for calculation

problem: distribution of point loads along length, located at ~midpoint between \( x_i \) and \( x_{i+1} \)

\[
\begin{align*}
nsta & := 7 \\
u \ell & := nsta - 1 \quad ll := 0 \\
j & := 0..ul - 1
\end{align*}
\]

\[
f' := \begin{pmatrix}
1 \\
2 \\
-3 \\
-3 \\
2 \\
1
\end{pmatrix} \quad x_s := \begin{pmatrix}
0 \\
1 \\
2 \\
3 \\
4 \\
5 \\
6
\end{pmatrix} \quad \begin{array}{l}
x_{x_j} := \frac{x_{s_j} + x_{s_{j+1}}}{2} \\
x = \begin{pmatrix}
0.5 \\
1.5 \\
2.5 \\
3.5 \\
4.5 \\
5.5
\end{pmatrix}
\end{array}
\]

\[
x := x_{s_0}, x_{s_0} + 0.01..x_{u \ell}
\]

\[
\text{shear}(x) := \sum_{i = ll}^{ul - 1} f'_i \left( x \geq x_{x_j} \right)
\]

\[
xs := x_0, x_0 + 0.01..x_{ul}
\]

\[
\text{location}
\]

\[
s_0 := 0 \quad i := 0..ul - 1 \quad s_{i+1} := s_i + f'_i
\]

\[
s = \begin{pmatrix}
0 \\
1 \\
3 \\
0 \\
-3 \\
-1 \\
0
\end{pmatrix} \quad x_s = \begin{pmatrix}
0 \\
1 \\
2 \\
3 \\
4 \\
5 \\
6
\end{pmatrix}
\]

\[
\text{shear}(x)
\]

\[
s
\]
bending moment algorithm: as above

\[ \text{bm} = \int_{0}^{x} \text{shear}(x) \, dx \]

\[ \text{bm}(x = x_{i1}) = \int_{0}^{x_{i1}} \text{shear}(x) \, dx = \int_{0}^{x_{i1}} \text{shear}(x) \, dx + \int_{x_{i1}}^{x_{i1}} \text{shear}(x) \, dx = \text{bm}(x = x_{i1}) + \int_{x_{i1}}^{x_{i1}} \text{shear}(x) \, dx \]

either model for shear =>

\[ \int_{x_{i1}}^{x_{i1}} \text{shear}(x) \, dx = \left( \text{shear}(x_{i1}) + \text{shear}(x_{i1}) \right) \frac{(x_{i1} - x_{i1})}{2} = \left( \frac{s_{i1} + s_{i1}}{2} \right) (x_{i1} - x_{i1}) \]

=> in index form:

\[ \text{bm}_0 := 0 \quad i := 1 \ldots 6 \quad \text{bm}_i := \text{bm}_{i-1} + \left( \frac{s_{i-1} + s_{i1}}{2} \right) (x_{i} - x_{i1}) \]

\[ x := x_{01} x_{01} + 0.1 \ldots x_{ul} \quad \text{bbm}(x) := \int_{0}^{x} \text{shear}(x) \, dx \quad \text{for comparison} \]
this algorithm is amenable to iterative schemes such as could be done in excel:

\[
\begin{bmatrix}
0 \\
0.5 \\
2.5 \\
0 \\
4 \\
2.5 \\
0 \\
0
\end{bmatrix}
\]

\[
\begin{bmatrix}
0 \\
1 \\
3 \\
0 \\
-3 \\
-1 \\
0
\end{bmatrix}
\]

\[
\begin{bmatrix}
0 \\
0.5 \\
2.5 \\
4 \\
2.5 \\
0.5 \\
0
\end{bmatrix}
\]

next we will develop the load from weight-buoyancy

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<th>load</th>
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<th>shear</th>
<th>bending</th>
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